# Future Secure Institute® Soil

1. The term soil mechanics was c (a) Rankine (b) Newton (c) T	<b>oined by</b> <mark>erzaghi</mark> (d) Newmarl	·k
2. The maximum size of grain od (a) 0.06 mm (b) 0.2 mm (c)	silt is about 0.5 mm (d) 1mm	
3. Cohesion less soil is: (a) Silt (b) Sand	(c) Clay	(d) None of the above
4. The property of a fluid which of (a) Viscosity (b) Surface tension		e to shearing stresses is called: (d) None of the above
5. Which of the following statem (a) Clay deposits are more porous (b) Presence of organic matter in (c) The change of moisture content (d) None of the above	sthan sand beds soil decreases the beari	
<b>6. Clay is an example of</b> (a) Aquifer (b) Aquitard	(c) Aquiclude	(d) Aquifuge
7. Black cotton soil is not suitable (a) Black color (b) Low bearing		
8. Clay is generally (a) Cohensive (b) Permeable	(c) Ha <mark>vi</mark> ng large p <mark>a</mark> rt	ticle size (d) None of the above
9. If the pores of a soil are compl (a) Wet soil (b) Dry soil	etely full of air only, the (c) Fully saturated s	
<b>10. The relation between porosit</b> (a) n = (1+e) /e (b) n = (1-e) /e	(c) n =(e/1+e)	
		te for the determination of the water content of soil? cium carbide method (d) Pycnometer method
12. If the voids of a soil mass are (a) Air entrained soil (b) Partia		l is termed as  Dry soil (d) None of the above
13. The ratio of volume of voids to (a) Air content (b) Porosit		
<b>14. Pycnomeyter is used to deter</b> (a) Water content and voids ratio (c) Water content and specific gra	(b) Specif	fic gravity and dry density e of the above
<b>15.</b> A sample of saturated clay ha (a) 1.283 (b) 0.438	as a porosity of <b>0.562,</b> to (c) 1.779	the void ratio of the clay is (d) 0.360

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16. If the sp.Gr.of the	soil is represented	by G and the vo	id ratio is e, the hydra	aulic gradient I is expressed as	
(a) G -1 /1+e	(b) G+1/1-e	(c)1-G/1+e	(d)1+G/1+e		
17. Pick up the incorre		_	y very high density as	compared to find grained soil	
(b) The dry density de				compared to find grained son	
(c) By compacting at a				% saturation	
(d) None of the above					
				The air content of that soil shall be:	
(a) 0.75	(b) 0.12	(c) 1.33	(d) 0.70		
19. Void ratio of a soil	is 0.9 its norosity s	shall he			
(a) 0.45	(b) 0.47	(c) 0.57	(d) 0.53		
(4) 0. 15	(3) 3117	(6) 0.37	(a) 0.00		
20. The volume of voi	ds to the total volu	me soil is knowr	n as		
(a) Porosity	(b) Void ratio	(c) Air ratio (d	d) Air content		
21. The value of comp				it is 50%, is	
(a) 0.028	(b) 0.28	(c) 0.36 (c	1) 0.036		
22.141					
		c (d) None of t		sticity index is reported as:	
(a) Negative (b) Zer	(c) Non-plasti	c (a) None of the	ne above		
23. The moisture cont	ent of soil below w	hich the soil vol	ume become constar	at is called the	
		hrinkage limit	(d) all of these		
(0)			Semi		
24. A negative value of	of the group index o	of a soil is report	ed as:		
(a) A positive value of the same magnitude dropping the negative sign (b) Zero					
(c) Negative value, as	GI may be negative	(d) (	GI is reported as non-	existent	
25. A soil with a plasti					
(a) Non-plastic (b)L	ow plastic (c)N	Medium plastic	(d)High plastic		
26. The ratio of plastic	city index and flow	index of soil is c	alled:		
	(b) Liquidity index	(c) Toughness		ncy index	
(a) our engan maex		(6) 1008111000		n Toorso	
27. The ratio Liquid lin	nit – Water conten	t / Plastic index	for a soil mass is calle	d Here	
(a) Liquidity index	(b) Shrinkage ratio	(c) Consistency	index (d) Toughnes	s index	
28. Relative density o		sand is approxir	mately equal to		
(a) 0.4 (b) 0.6 (c	<mark>:) 0.95</mark> (d) 1.20				
	•-				
29. Specific gravity ha		/-\ -/ /-I\ IX	N1 / 3		
(a) N/m³ (b) No uni	it-dimensionless	(c) g/cc (d) K	N/m³		
20. When the plactic l	imit of a soil is gros	star than the lieu	uid limit than the nla	sticity index is reported as:	
(a) 0 (b) Non-pl	_	_	legative	sticity maex is reported as.	
(b) Non-pi	astic	(C) 1 (U) N	Court		
31. A pycnometer is u	sed to determine:				
a) Water content and					
a) water content and	void ratio	(b) Specific gra	vity and dry density		

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32. Gravel and sand belongs to the following category of soils: (a) Expansive (b) Marine (c) Alluvial (d) Cohesive
33. A soil has an average particles size of 2.0 mm. It is predominantly: (a) Gravel (b) Sand (c) Silt (d) Clay
24. The coefficient of neumochility of clay is not move than
<b>34.</b> The coefficient of permeability of clay is not more than a) 1 cm/sec (b) 1 X 10 <sup>-2</sup> cm/sec (c) 1 X 10 <sup>-4</sup> cm/sec (d) 1 X 10 <sup>-6</sup> cm/sec
a) I chiyace (b) I x 10 chiyace (c) I x 10 4 chiyace (d) I x 10 0 chiyace
35. Stoke is the unit of (a) Dynamic viscosity (b) Kinematic (c) Specific volume (d) Specific weight
36. Physic properties which influence permeability are:
(a) Viscosity only (b) Unit weight only (c) Both viscosity and unit weight (d) None of the above
37. The meniscus and dispersing agent corrections, in the hydrometer analysis, respectively are
(a) Positive and negative (b) Negative and positive
(c) Positive and positive (d) Negative and negative
38. An overall value of the coefficient of permeability of a soil deposit for a large area may be determined by:
(a) Constant head permeability test (b) Variable head permeability test
(c) Pumping out tests (d) Pumping in tests
39. The expression for the discharge (Q) through a flow net isotropic soil is given by:
(a) $Q = KH \times NF/ND$ (b) $Q = KH \vee NF/ND$ (c) $Q = KH (NF/ND)^2$ (d) $Q = KH (NF/ND)^3$
Rocksona Raccona
40. Standard penetration resistance in very stiff clays lies between:
(a) Lies b/w 2 and 4 (b) Lies b/w 4 and 8 (c) Lies b/w 8 and 15 (d) Lies b/w 15 and 30
41. For determining the ultimate heaving and its of all the second are of access heaving plate used in
41. For determining the ultimate bearing capacity of soil the recommended size of square bearing plate used in plate load test is 30-75 cm with a minimum thickness of:
(a) 20 mm (b) 5 mm (c) 50 mm (d) None of these
(a) 20 min (b) 3 min (c) 30 min (a) None of these
42. The ultimate consolidation settlement of a structure resting on a soil
(a) Decreases with the increase in the initial voids ratio
(b) Decreases with the decrease in the plastic limit
(c) Increases with the increase in the initial voids ratio
(d) Increase with the decrease in the porosity of the soil
43. The coefficient of consolidation is used for evaluating:
(a) Stress in the soil (b) Total settlement
(c) Over consolidation ratio (d) Time rate of settlement
44. The rate of consolidation.
(a) Increase with decrease in temperature (b) Increase with increase in temperature
(c) Is independent of temperature (d) All the above
45. For routine consolidation test is laboratory, the thickness of the specimen is
(a) 10 mm (b) 20 mm (c ) 40 mm ( <mark>d) 60 mm</mark>
46. The consolidation time for soils
(a) Increase with increasing compressibility (b) Decrease with the increasing permeability
(c) Is independent of the stress change (d) All of the above

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#### 47. The unit of the coefficient of consolidation is

(a) cm<sup>2</sup>/sec (b) cm<sup>3</sup>/sec (c) gm/cm<sup>2</sup>/sec (d) gm-cm/sec

## 48. The shearing strength of a cohesion less soil depends on:

(a) Dry density (b) Rate of loading (c) Confining pressure (d) All of the above

## 49. The strength of a soil is usually identified by

(a) Direct tensile stress (b) Direct compressive stress

(c) Ultimate shear stress (d) Effective stress

#### 50. The useful method of finding the shear strength of very plastic cohesive soils is by means of

(a) Cone test b) Penetration test (c) Vane shear test (d) Torsional shear test



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